

B.M.S. College of Engineering, Bengaluru-560019

Autonomous Institute Affiliated to VTU

August 2024 Supplementary Examinations

Programme: B.E.

Branch: Civil Engineering

Course Code: 19CV4PCGTE

Course: Geotechnical Engineering -I

Semester: IV

Duration: 3 hrs.

Max Marks: 100

Instructions: 1. Answer any FIVE full questions, choosing one full question from each unit.
2. Missing data, if any, may be suitably assumed.

Important Note: Completing your answers, compulsorily draw diagonal cross lines on the remaining blank pages. Revealing of identification, appeal to evaluator will be treated as malpractice.

		UNIT – I				CO	PO	Marks										
1	a)	Differentiate between residual soils and transported soil with examples				CO 1	PO1	04										
	b)	The dry unit weights of a sand in the loosest and densest state are respectively 1.36 and 2.18g/cc. Assuming the specific gravity of solids as 2.67, determine the relative density of sand with porosity of 30%				CO 2	PO2	08										
	c)	A partially saturated soil sample obtained from an earth fill has a moisture content of 22% and bulk unit weight of 2g/cc. Assuming specific gravity of solids as 2.7 and density of water as 1g/cc, compute i. degree of saturation ii. void ratio iii. if subsequently the soil gets saturated, determine its unit weight				CO 2	PO2	08										
		UNIT – II																
2	a)	The following observations were obtained from a liquid limit test of a soil <table><tr><td>No. of blows (N)</td><td>10</td><td>20</td><td>31</td><td>40</td></tr><tr><td>Water content (w)</td><td>82.0%</td><td>74.3%</td><td>68.0%</td><td>65.0%</td></tr></table> Two tests for plastic limit were also performed, which gave values of 28.0% and 29.0% respectively. i. Plot the flow curve and ii. Determine the liquid limit, plasticity index, flow index and toughness index				No. of blows (N)	10	20	31	40	Water content (w)	82.0%	74.3%	68.0%	65.0%	CO 2	PO2	12
No. of blows (N)	10	20	31	40														
Water content (w)	82.0%	74.3%	68.0%	65.0%														
	b)	Discuss the various field identification tests				CO 1	PO1	08										
		OR																

3	a)	<p>The following table gives the results of the particle size analysis, performed on 3 different soil samples A, B and C.</p> <table><tr><th rowspan="2">% Finer</th><th colspan="3">Diameter of particles in mm</th></tr><tr><th>Soil A</th><th>Soil B</th><th>Soil C</th></tr><tr><td>90</td><td>1.20</td><td>0.075</td><td>0.0017</td></tr><tr><td>80</td><td>0.30</td><td>0.068</td><td>0.0011</td></tr><tr><td>70</td><td>0.15</td><td>0.062</td><td>0.00072</td></tr><tr><td>60</td><td>0.043</td><td>0.056</td><td>0.00050</td></tr><tr><td>50</td><td>0.019</td><td>0.050</td><td>0.00036</td></tr><tr><td>40</td><td>0.009</td><td>0.045</td><td>0.00030</td></tr><tr><td>30</td><td>0.0045</td><td>0.040</td><td>0.00023</td></tr><tr><td>20</td><td>0.0022</td><td>0.029</td><td>0.00016</td></tr><tr><td>10</td><td>0.0012</td><td>0.020</td><td>0.00011</td></tr></table> <p>The following additional data has been obtained for the above mentioned samples:</p> <table><tr><th>Soil</th><th>Liquid limit (%)</th><th>Plastic limit (%)</th></tr><tr><td>A</td><td>31</td><td>27.1</td></tr><tr><td>B</td><td>52</td><td>35</td></tr><tr><td>C</td><td>22</td><td>Non Plastic</td></tr></table> <p>Plot the grain size distribution curves, and classify the soils A, B and C according to IS classification system. What percentage of gravel, sand, silt and clay are present in each soil.</p>	% Finer	Diameter of particles in mm			Soil A	Soil B	Soil C	90	1.20	0.075	0.0017	80	0.30	0.068	0.0011	70	0.15	0.062	0.00072	60	0.043	0.056	0.00050	50	0.019	0.050	0.00036	40	0.009	0.045	0.00030	30	0.0045	0.040	0.00023	20	0.0022	0.029	0.00016	10	0.0012	0.020	0.00011	Soil	Liquid limit (%)	Plastic limit (%)	A	31	27.1	B	52	35	C	22	Non Plastic	CO 2	P02	14
% Finer	Diameter of particles in mm																																																											
	Soil A	Soil B	Soil C																																																									
90	1.20	0.075	0.0017																																																									
80	0.30	0.068	0.0011																																																									
70	0.15	0.062	0.00072																																																									
60	0.043	0.056	0.00050																																																									
50	0.019	0.050	0.00036																																																									
40	0.009	0.045	0.00030																																																									
30	0.0045	0.040	0.00023																																																									
20	0.0022	0.029	0.00016																																																									
10	0.0012	0.020	0.00011																																																									
Soil	Liquid limit (%)	Plastic limit (%)																																																										
A	31	27.1																																																										
B	52	35																																																										
C	22	Non Plastic																																																										
	b)	What are the assumptions and limitations of Stoke’s law ?	CO 1	P01	06																																																							
		UNIT - III																																																										
4	a)	Discuss the major clay minerals with neat sketches	CO 1	P01	12																																																							
	b)	The water table in an area is 5m below the natural ground level. The ground strata consists of a soil of very fine sand having an average void ratio of 0.65 up to 13m depth. Above the water table, the sand has an average degree of saturation of 50%. Calculate the effective stress on a horizontal plane at a depth of 11m below the ground level. What will be this effective stress and increase in stress if the soil gets saturated by capillarity up to a height of 1m above the water table. Assume G = 2.65	CO 2	P02	08																																																							
		UNIT – IV																																																										
5	a)	A horizontal stratified soil deposit consists of three layers, each uniform in itself. The permeabilities of the layers are 8×10^{-4} cm/s, 50×10^{-4} cm/s and 15×10^{-4} cm/s; and their thicknesses are 6m, 3m and 12m respectively. Find the effective average permeability of the deposit in horizontal and vertical directions.	CO 2	P02	08																																																							
	b)	What are the effects of compaction on the soil properties	CO 1	P01	06																																																							

	c)	Discuss the factors affecting permeability of soil	CO 1	PO1	06																
		OR																			
6	a)	A sand sample is tested in a constant head permeameter 11.7cm high and 10.2 cm in diameter. The quantity of water passing through the sample under an effective head of 10cm, for a period of 90 seconds was measured to be 600ml. Determine the coefficient of permeability.	CO 2	PO2	06																
	b)	<div>A Proctor compaction test was conducted on a soil sample, and the following observations were made:<table><tr><td>Water content (%)</td><td>8</td><td>11.5</td><td>14.5</td><td>17.5</td><td>19.5</td><td>21.5</td></tr><tr><td>Weight of wet soil (kg)</td><td>1.70</td><td>1.90</td><td>2.00</td><td>1.98</td><td>1.95</td><td>1.92</td></tr></table><div>If the volume of the mould used was 950cc and the specific gravity of the soil was 2.65,<div><div>i. Draw the dry density vs. Moisture content curve</div><div>ii. Also plot the 100% and 80% saturation lines</div><div>iii. Find the maximum dry density and optimum moisture content</div><div>iv. Also find the degree of saturation at OMC</div></div></div></div>	Water content (%)	8	11.5	14.5	17.5	19.5	21.5	Weight of wet soil (kg)	1.70	1.90	2.00	1.98	1.95	1.92	CO 2	PO2	14		
Water content (%)	8	11.5	14.5	17.5	19.5	21.5															
Weight of wet soil (kg)	1.70	1.90	2.00	1.98	1.95	1.92															
		UNIT – V																			
7	a)	Explain triaxial shear test with different drainage conditions	CO 1	PO1	10																
	b)	<div>Undrained triaxial compression tests are carried out to failure on three specimens of clayey silt, with pore pressure measurements as shown below<table><tr><td>Specimen No.</td><td>Major principal stress (kg/cm²)</td><td>Minor principal stress (kg/cm²)</td><td>Pore pressure (kg/cm²)</td></tr><tr><td>1</td><td>1.57</td><td>0.17</td><td>0.12</td></tr><tr><td>2</td><td>2.04</td><td>0.44</td><td>0.20</td></tr><tr><td>3</td><td>2.25</td><td>0.55</td><td>0.22</td></tr></table><div>By solving analytically<div><div>i. Determine the shear parameters constituting the shear strength of the soil</div><div>ii. Also determine how much increase or decrease in these parameters would be computed if the pore pressures were neglected</div></div></div></div>	Specimen No.	Major principal stress (kg/cm ²)	Minor principal stress (kg/cm ²)	Pore pressure (kg/cm ²)	1	1.57	0.17	0.12	2	2.04	0.44	0.20	3	2.25	0.55	0.22	CO 2	PO2	10
Specimen No.	Major principal stress (kg/cm ²)	Minor principal stress (kg/cm ²)	Pore pressure (kg/cm ²)																		
1	1.57	0.17	0.12																		
2	2.04	0.44	0.20																		
3	2.25	0.55	0.22																		
