

U.S.N.

B.M.S. College of Engineering, Bengaluru-560019

Autonomous Institute Affiliated to VTU

January / February 2025 Semester End Main Examinations

Programme: B.E.

Branch: Civil Engineering

Course Code: 20CV6PCIWR

Course: Irrigation and Water Resources

Semester: VI

Duration: 3 hrs.

Max Marks: 100

- Instructions:**
1. Answer 5 full questions choosing one full question from unit.
 2. Assume missing data suitably.
 3. Draw neat sketches wherever necessary.
 4. Additional normal graph sheets may be supplied.

Important Note: Completing your answers, compulsorily draw diagonal cross lines on the remaining blank pages. Revealing of identification, appeal to evaluator will be treated as malpractice.

		UNIT - I	CO	PO	Marks																																																											
1	a)	Interpret Horton’s engineering representation of the global hydrologic cycle process with a neat sketch.	CO1	PO1	05																																																											
	b)	A lake had a water surface elevation of 109.950 m above datum at the beginning of certain month. In that month the lake reserved an average inflow of 7.05 cumecs (m³/s) from surface runoff sources. In the same period outflow from the lake have an average value of 6.75 cumecs. Further in that month the lake received a rainfall of 156 mm and evaporation from lake surface was estimated at 8.10 cm. Write the water budget equation for lake and calculate the water surface elevation of the lake at the end of the month. The average lake surface area may be taken as 4500 hectares. Assume that there is no contribution to or from ground water storage.	CO1	PO1	07																																																											
	c)	The recorded rainfall at 4 rain gauge stations in a catchment for a period of 9 years is given below. Estimate the missing rainfall at station X in the year 2013 by normal ratio method. <table><tr><th rowspan="2">Year</th><th colspan="4">Annual Rainfall in cm at 4 rain gauge stations</th></tr><tr><th>A</th><th>B</th><th>C</th><th>X</th></tr><tr><td>2010</td><td>50.2</td><td>56.8</td><td>60.8</td><td>52.1</td></tr><tr><td>2011</td><td>75.8</td><td>74.8</td><td>77.2</td><td>78.3</td></tr><tr><td>2012</td><td>103.5</td><td>100.9</td><td>105.4</td><td>104.5</td></tr><tr><td>2013</td><td>42.6</td><td>40.8</td><td>45.9</td><td>Missing Data</td></tr><tr><td>2014</td><td>56.9</td><td>52.4</td><td>60.7</td><td>63.9</td></tr><tr><td>2015</td><td>77.9</td><td>69.7</td><td>80.9</td><td>75.9</td></tr><tr><td>2016</td><td>68.5</td><td>52</td><td>65.8</td><td>70.5</td></tr><tr><td>2017</td><td>66.8</td><td>68.8</td><td>67.5</td><td>70.9</td></tr><tr><td>2018</td><td>92.5</td><td>88</td><td>95.8</td><td>90.4</td></tr><tr><td>2019</td><td>100.2</td><td>89.5</td><td>103.8</td><td>101.6</td></tr></table>	Year	Annual Rainfall in cm at 4 rain gauge stations				A	B	C	X	2010	50.2	56.8	60.8	52.1	2011	75.8	74.8	77.2	78.3	2012	103.5	100.9	105.4	104.5	2013	42.6	40.8	45.9	Missing Data	2014	56.9	52.4	60.7	63.9	2015	77.9	69.7	80.9	75.9	2016	68.5	52	65.8	70.5	2017	66.8	68.8	67.5	70.9	2018	92.5	88	95.8	90.4	2019	100.2	89.5	103.8	101.6	CO1	PO1	08
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2	a)	Illustrate the importance of drainage point in relation to water budget	CO1	PO1	05																																																											

		equation of the catchment.																																										
	b)	The normal annual precipitation of five raingauge stations P, Q, R, S and T are respectively 125, 102, 76, 113 and 137 cm. During a particular storm, the precipitation recorded at stations P, Q, R, and S are 13.2, 9.2, 6.8 and 10.2 cm respectively. The instrument at station T was inoperative during that storm. Estimate the rainfall at station T during that storm.	CO1	PO1	05																																							
	c)	In a catchment, whose shape can be approximated by a pentagon, four raingauge stations are present. The coordinates of the corners of the catchment that define its boundaries and the coordinates of the four raingauge stations are given below. Also given are the annual rainfall recorded at the four stations in the year 2005. Determine the average annual rainfall over the catchment in that year by the Thiessen mean method. <table border="1"><tr><td colspan="2">Distances are in Km.</td><td colspan="5">Corner a is the origin of co-ordinates.</td></tr><tr><td rowspan="2">Catchment Boundary</td><td>Corner</td><td>a</td><td>b</td><td>c</td><td>d</td><td>e</td></tr><tr><td>Co-ordinates</td><td>(0,0)</td><td>(120,0)</td><td>(120,80)</td><td>(60,140)</td><td>(0,80)</td></tr><tr><td rowspan="3">Rain gauge Station</td><td>Station</td><td>P</td><td>Q</td><td>R</td><td colspan="2">S</td></tr><tr><td>Co-ordinates</td><td>(40,20)</td><td>(80,20)</td><td>(80,60)</td><td colspan="2">(40,80)</td></tr><tr><td>Annual Rainfall, cm</td><td>120</td><td>110</td><td>100</td><td colspan="2">125</td></tr></table>	Distances are in Km.		Corner a is the origin of co-ordinates.					Catchment Boundary	Corner	a	b	c	d	e	Co-ordinates	(0,0)	(120,0)	(120,80)	(60,140)	(0,80)	Rain gauge Station	Station	P	Q	R	S		Co-ordinates	(40,20)	(80,20)	(80,60)	(40,80)		Annual Rainfall, cm	120	110	100	125		CO1	PO2	10
Distances are in Km.		Corner a is the origin of co-ordinates.																																										
Catchment Boundary	Corner	a	b	c	d	e																																						
	Co-ordinates	(0,0)	(120,0)	(120,80)	(60,140)	(0,80)																																						
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	Co-ordinates	(40,20)	(80,20)	(80,60)	(40,80)																																							
	Annual Rainfall, cm	120	110	100	125																																							
		UNIT - II																																										
3	a)	Define evaporation. List the factors affecting evaporation. Illustrate the ISI standard pan for measuring evaporation.	CO2	PO1	10																																							
	b)	The following observations were made from a double ring infiltrometer. Plot the infiltration capacity curve and determine the average infiltration rate for the first 25 minutes of the experiment.	CO2	PO1	10																																							
		<table border="1"><tr><td>Time (hr)</td><td>5</td><td>10</td><td>15</td><td>25</td><td>45</td><td>60</td><td>75</td><td>90</td><td>110</td><td>130</td></tr><tr><td>Cumulative infiltration depth (cm)</td><td>1.72</td><td>3.2</td><td>3.97</td><td>5.56</td><td>7.35</td><td>8.3</td><td>9.43</td><td>10.2</td><td>11.28</td><td>12.36</td></tr></table>	Time (hr)	5	10	15	25	45	60	75	90	110	130	Cumulative infiltration depth (cm)	1.72	3.2	3.97	5.56	7.35	8.3	9.43	10.2	11.28	12.36																				
Time (hr)	5	10	15	25	45	60	75	90	110	130																																		
Cumulative infiltration depth (cm)	1.72	3.2	3.97	5.56	7.35	8.3	9.43	10.2	11.28	12.36																																		
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4	a)	A storm over a catchment area of 5 sqkm had a duration of 14 hrs. The mass curve of rainfall of the storm is as follows: <table border="1"><tr><td>Time from start of the storm (hr)</td><td>0</td><td>2</td><td>4</td><td>6</td><td>8</td><td>10</td><td>12</td><td>14</td></tr><tr><td>Accumulated rainfall (cm)</td><td>0</td><td>0.6</td><td>2.8</td><td>5.2</td><td>6.6</td><td>7.5</td><td>9.2</td><td>9.6</td></tr></table> If the ϕ (phi) index is 0.4cm/hr, determine the effective rainfall hyetograph and the volume of direct runoff from the catchment due to the storm.	Time from start of the storm (hr)	0	2	4	6	8	10	12	14	Accumulated rainfall (cm)	0	0.6	2.8	5.2	6.6	7.5	9.2	9.6	CO2	PO2	10																					
Time from start of the storm (hr)	0	2	4	6	8	10	12	14																																				
Accumulated rainfall (cm)	0	0.6	2.8	5.2	6.6	7.5	9.2	9.6																																				
	b)	Illustrate the various components of a hydrograph. Elaborate on methods of base flow separation and computation of the unit hydrograph ordinates.	CO2	PO1	10																																							

		UNIT - III																																													
5	a)	Explain what is meant by stream gauging and list the various methods available for stream gauging. With a neat sketch, explain the area-velocity method of stream gauging.								CO3	PO1	10																																			
	b)	The following observations were made at gauging site while recording stream gauge data. The rating equation of the current meter is $V=0.31N_s+0.75$ m/s, where N_s is in revolutions per second. Calculate the discharge in the stream.								CO3	PO1	10																																			
		Distance from left water edge (m)	0	1.0	3.0	5.0	7.0	9.0	11.0	12.0																																					
		Depth (m)	0	1.4	2.2	2.0	2.5	1.8	1.0	0																																					
		Revolutions of the current meter at a depth of 0.6m	0	35	63	52	108	39	30	0																																					
		Duration of observations (s)	0	98	100	100	150	95	100	0																																					
		OR																																													
6	a)	The following data were collected during a stream-gauging operation in a river. Compute the discharge. <table><tr><td colspan="2">Distance from left water edge(m)</td><td>0</td><td>1.5</td><td>3</td><td>4.5</td><td>6</td><td>7.5</td><td>9</td></tr><tr><td colspan="2">Depth (m)</td><td>0</td><td>1.3</td><td>2.5</td><td>1.7</td><td>10</td><td>0.4</td><td>0</td></tr><tr><td rowspan="2">Velocity (m/s)</td><td>At 0.2 depth</td><td>0</td><td>0.6</td><td>0.9</td><td>0.7</td><td>0.6</td><td>0.4</td><td>0</td></tr><tr><td>At 0.8 depth</td><td>0</td><td>0.4</td><td>0.6</td><td>0.5</td><td>0.4</td><td>0.3</td><td>0</td></tr></table>								Distance from left water edge(m)		0	1.5	3	4.5	6	7.5	9	Depth (m)		0	1.3	2.5	1.7	10	0.4	0	Velocity (m/s)	At 0.2 depth	0	0.6	0.9	0.7	0.6	0.4	0	At 0.8 depth	0	0.4	0.6	0.5	0.4	0.3	0	CO3	PO1	10
Distance from left water edge(m)		0	1.5	3	4.5	6	7.5	9																																							
Depth (m)		0	1.3	2.5	1.7	10	0.4	0																																							
Velocity (m/s)	At 0.2 depth	0	0.6	0.9	0.7	0.6	0.4	0																																							
	At 0.8 depth	0	0.4	0.6	0.5	0.4	0.3	0																																							
	b)	During a flood flow, depth of water in a 12 m wide rectangular channel was found to be 3.1 m and 2.9 m at two sections 300 m apart. The drop in the water-surface elevation was found to be 0.20 m. Assuming Manning's coefficient to be 0.025, estimate the flood discharge through the channel.								CO3	PO1	10																																			
		UNIT - IV																																													
7	a)	Illustrate a typical Area-Elevation-Capacity curve and discuss its relevance in reservoir planning and operations.								CO4	PO1	06																																			
	b)	For the following conditions, select the suitable method of irrigation and justify the same with schematic diagrams and technical features. i) Hilly terrain with steep slope ii) Arecanut Plantations with silt soil								CO4	PO1	06																																			
	c)	Discuss the engineering and hydrological investigations essential during the reservoir planning								CO4	PO1	08																																			
		OR																																													
8	a)	Differentiate between free flooding and border flooding with the help of diagrams.								CO4	PO1	05																																			
	b)	Discuss the disadvantages of irrigation projects in the Indian context.								CO4	PO1	05																																			
	c)	Comment on functionality of various storage zones of reservoir with the help of neat diagram.								CO4	PO1	10																																			
		UNIT - V																																													
9	a)	A water course has culturable command area (CCA) of 2650 hectares, out of which the intensities of irrigation for perennial sugarcane and rice crops are 20% and 40% respectively. The duty for these crops at the head of water course are 1500 hectares/cumec and 1850 hectares/cumec								CO5	PO1	10																																			

		respectively. Determine the discharge required at the head of the water course if the peak demand is 140% of the average requirement.																							
	b)	<p>The base period, intensity of irrigation and duty of water for various crops under a canal system are given. Determine the reservoir capacity in Million cubic meters if culturable command area is 6000 hectares. The conveyance loss is 12% and reservoir losses are 10%.</p> <table><tr><td>Crop</td><td>Base Period (days)</td><td>Duty of field (Hectares/cumec)</td><td>Intensity of Irrigation</td></tr><tr><td>Wheat</td><td>120</td><td>1800</td><td>25%</td></tr><tr><td>Sugar cane</td><td>180</td><td>1700</td><td>25%</td></tr><tr><td>Rice</td><td>120</td><td>800</td><td>35%</td></tr><tr><td>Maize</td><td>90</td><td>900</td><td>15%</td></tr></table>	Crop	Base Period (days)	Duty of field (Hectares/cumec)	Intensity of Irrigation	Wheat	120	1800	25%	Sugar cane	180	1700	25%	Rice	120	800	35%	Maize	90	900	15%	CO5	PO2	10
Crop	Base Period (days)	Duty of field (Hectares/cumec)	Intensity of Irrigation																						
Wheat	120	1800	25%																						
Sugar cane	180	1700	25%																						
Rice	120	800	35%																						
Maize	90	900	15%																						
		OR																							
10	a)	<p>The following data is observed in an irrigation field:</p> <ul style="list-style-type: none">- Field Capacity of soil =27%- Permanent Wilting Point= 14%- Dry Density of soil = 15 kN/m³- Effective depth of root zone= 75 cm- Daily consumptive use of water for the given crop =11 mm <p>Determine the frequency of irrigation to ensure efficient irrigation.</p>	CO5	PO2	10																				
	b)	Differentiate between Gross Irrigation Requirement and Net Irrigation Requirement. Enumerate the steps involved in calculating Gross Irrigation Requirement for a command area	CO5	PO1	10																				
